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Articles are published in Uzbek, Russian, and English, ensuring a wide-reaching audience and fostering cross-cultural academic exchange. As a beacon of academic excellence, the "Journal of Transport" continues to serve as a vital conduit for knowledge dissemination, collaboration, and innovation in the transport sector and related fields.

Analysis of methods for calculating long-span multi-span structures for seismic stresses

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Abstract:

The consideration of structural length when calculating seismic force effects is one of the pressing issues that interests today's engineers. Numerous studies conducted on bridges demonstrate that their supports oscillate asynchronously under seismic forces, indicating the need for a new approach in assessing earthquake forces. The asynchronous oscillations of supports lead to both changes in inertial seismic loads and the formation of quasistatic forces associated with the relative displacement of supports. This presents a critical problem for statically indeterminate structures, which are highly sensitive to support displacements. Based on the experience of past earthquakes, the building codes of most countries, including Russia, prohibit the construction of hingeless arched domes on rocky soils in seismically hazardous areas. Although the seismic loads on these domes are not significant, they are often prone to damage as a result of seismic impact. Thus, the analysis of damage in multi-span bridge structures indicates the need for specific development of calculation methods that take into account various types of support vibrations. This article analyzes research conducted specifically on this topic.

Keywords:

asynchronous oscillation, acceleration field, quasi-static displacements, calculated seismic forces, degree of correlation, coherence coefficient

Uzunligi katta bo'lgan ko'p oraliqli konstruksiyalarni seysmik kuchlanishlarga hisoblash uslublarining tahlili

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Annotatsiya:

Inshootlarni seysmik kuchlar ta'siriga hisoblashda ularning uzunligini hisobga olish masalasi bugungi kun muhandislarini qiziqtirgan dolzarb masalalardan biridir. Ko'priklarda o'tkazilgan ko'plab tadqiqotlar ularning tayanchlari seysmik kuchlar ta'sirida nosinxron tebranishini, zilzila kuchini baholashda yangi yondashuv zarurligini ko'rsatadi. Tayanchlar tebranishlarining nosinxronligi ham inersion seysmik yuklanishlarning o'zgarishiga, ham tayanchlarni bir-biriga nisbatan siljishi bilan bog'liq kvazistatik kuchlarning hosil bo'lishiga olib keladi va bu tayanchlarning siljishiga o'ta sezgir bo'lgan statik noaniq konstruksiyalar uchun dolzarb muammo hisoblanadi. Bo'lib o'tgan zilzilalar tajribasiga asoslangan holda, aksariyat davlatlar, shu jumladan Rossiya me'yorlarida, masalan, seysmik jihatdan xavfli hududlarning qoyatoshli gruntlarida sharnirsiz ravoqli gumbazlarni qurish ta'qiqlanadi. Bu gumbazlarga tushadigan seysmik yuklanishlar katta bo'lmasa ham, ko'pincha ularning seysmik ta'sir natijasida shikastlanishi bilan bog'liq. Shunday qilib, ko'p oraliqli ko'priklarda inshootlaridagi shikastlarning tahlili ularni hisoblash usullarini tayanchlarning turli xildagi tebranishlarini hisobga olgan holda o'ziga xos rivojlantirish kerakligini ko'rsatadi. Ushbu maqolada aynan shu mavzuda bajarilgan tadqiqotlar tahlil qilingan.

Kalit so'zlar:

nosinxron tebranish, tezlanishlar maydoni, kvazistatik siljishlar, hisobiy seysmik kuchlar, korrelyatsiya darajasi, kogerentlik koeffitsiyenti

1. Kirish

Inshootlar uzunligini hisobga olish bo'yicha dastlabki jiddiy tadqiqot ishlari zilzilabardoshlik nazariyasi sohasida yetakchi mutaxassislardan M.F. Barshteyn va I.L. Korchinskiylar [1, 2] tomonidan bajarilgan, o'tgan asrning 70-yillarida A.B. Grossman [3], E.F. Pak [4], A.A. Petrov [5], A.Ter-Kyuregyan [6-8], Dj.L. Bogdanov, Dj.I. Goldberg va A.Dj. Shiff [9], X.L. Vong [10] va boshqa mutaxassislar davom ettirishgan.

Massiv inshootlar uchun nosinxronlik effekti eng katta tezlanish o'rniga inshoot tagidagi tezlanishning o'rtaqcha qiymatini hisobga olish zarurati yuzaga kelishi orqali namoyon bo'ladi. Bu holat 1-rasmda ifodalangan. Shuningdek, bunday inshootlarda burovchi va aylantiruvchi tebranishlarning yuzaga kelishi bilan bog'liq qo'shimcha effektni ham e'tiborga olish zarur.

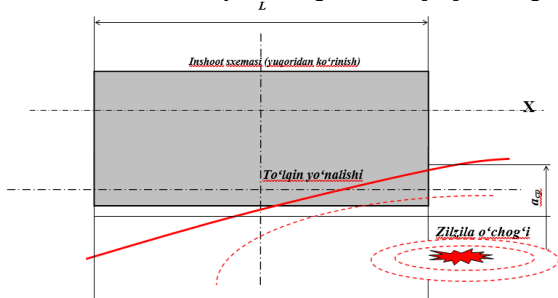
A.B. Grossman [3] va I.L. Korchinskiylarning ishlari [11] massiv inshootdagi tezlanishlar maydoni va qo'shimcha moment yuklanishlarning o'rtaqcha qiymati baholangan. Hozirgi vaqtda bu turga mansub baholashlarni yanada

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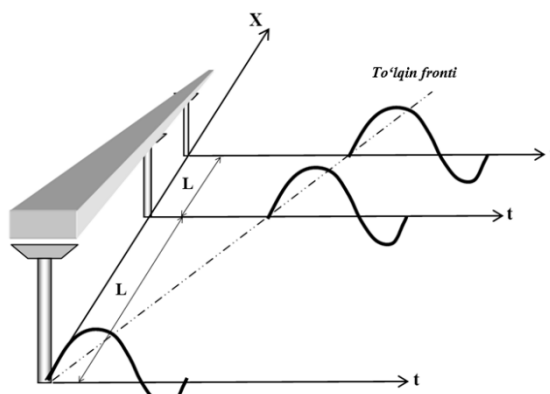


aniqlashtirish masalasiga katta e'tibor qaratilmoqda. Masalan, [12] ishda Tadotsu-Cho (Yaponiya) nomli dengiz atrofi zonasida 90×45 m o'lchamli temirbeton plitasida amalga oshirilgan tajriba bayon etilgan. Bu eksperiment natijasida poydevor tagidagi eng katta va o'rtacha tezlanishlar orasidagi farq 20 % ni tashkil etishi aniqlandi. Ushbu masalaga oid mukammal nazariy ishlanmalar Y.P. Nazarov va YE.V. Poznyaklarning ishlarida [13] keltirilgan.



1-rasm. Inshoot uzunligi bo'yicha tezlanishlar maydoni notekislik effektining namoyon bo'lish sxemasi

O'tgan asrning 70-yillaridan boshlab Sobiq Ittifoq va MDH olimlari (Gruziyada K.S. Zavriyev, O'zbekistonda V.T. Rasskazovskiy, Armanistonda E.Y. Xachiyanlarning ilmiy maktablari, V.A.Kucherenko nomidagi SNIISK, A.P. Sinitsin, A.A. Petrov, V.T. Rasskazovskiy, A. Ter-Kyurigyan, V.M. Fomin, A.M.Uzdin va L.N. Gimannlarning, chet el olimlari M.F. Barshteyn, E.F. Pak va boshqa ko'plab atoqli olimlarning amalga oshirgan ishlari natijasida zilzilabardoshlik nazariyasida to'lqin yo'nalishi rivojlantirilmoqda. Xususan, Tehron universiteti olimlari G.R. Nouri, M. Shahrouzi va E. Farhadi tomonidan olib borilgan tadqiqot ishida [14] uzun inshootlar tayanchlari ostidagi gruntlarning differensial to'lqinlar natijasida nosinxron ko'chishi ko'prik tayanchlaridagi bo'ylama kuch, ko'ndalang kuch va eguvchi moment qiymatlarining mos ravishda 1,87; 1,8 va 1,97 marta ortishiga; bundan tashqari, qurilish maydonchasidagi grunt qatlamining turli jinsli ekanligi ko'prik tayanchlaridagi bo'ylama kuch miqdorining 55 % ga ortishiga sabab bo'lishi aniqlangan. [15-16] ishlarda ko'p tayanchli sanoat va transport inshootlarining tebranishlari ko'rib chiqilgan. Jumladan, [17] ishda bir qavatli ko'p tayanchli sanoat binosining tebranish tenglamalari ishlab chiqilgan, [15] ishda esa quvuro'tkazgichdagi seysmik tebranishlar, [16] ishda esa uzun quvuro'tkazgichning seysmik tebranishlari ko'rib chiqilgan. Bu ishlarda tebranishlar so'nishi bitta o'rtacha qiymat ko'rinishida ichki ishqalanish koeffitsiyentidan foydalangan holda taqriban hisobga olingan yoki umumam hisobga olinmagan, ta'sir esa yuguruvchi muzlagan to'lqin ko'rinishida modellastiriladi (2-rasm).



2-rasm. Ko'prik tayanchiga yuguruvchi muzlagan to'lqinlarning ta'siri

2. Tadqiqot metodologiyasi

Yuqorida qayd etib o'tilgan tadqiqotlar natijasida hozirgi vaqtda e'tiborga olinishi muhim bo'lgan ikkita katta ahamiyatga ega qonuniyat aniqlandi. A.N.Birbrayerning monografiyasida [18] aniq va ravshan ta'riflab berilgan bu qonuniyatlarning mohiyati quyidagilardan iborat:

– *tayanchlar tebranishlari nosinxronligining hisobga olinishi hisobiy seysmik kuchlarning kamayishiga olib keladi;*

– *statik noaniq konstruksiyalarda tayanchlarning bir-biriga nisbatan kvazistatik siljishlari bilan bog'liq qo'shimcha kuchlar vujudga keladi.*

Hozirgi vaqtda aksariyat mutaxassislar tomonidan zilzilalar akselerogrammalari bo'yicha bajariladigan hisoblashlar aniqroq deb qabul qilinadi va aksariyat tadqiqot ishlarida shu usuldan foydalanishni ma'qul ko'riladi. Jumladan, Italiyalik olim G. Tecchio tomonidan olib borilgan tadqiqot ishida [19] ko'p oraliqli temirbeton ko'priklarni zilzila ta'siriga hisoblashda seysmik kuchlar ta'sirini baholash uchun kuchning spektral zichlik funksiyasi (PSD) va kogerentlik funksiyasi (CF) ni hisobga olgan holda akselerogrammalarda chastota tarkibining o'zgarishi tasvirlangan. Ammo ko'p tayanchli konstruksiyalarni zilzilalarning akselerogrammalari orqali hisoblashni oqilona usul deb hisoblab bo'lmaydi. Ko'p tayanchli konstruksiyalarni zilzilalar akselerogrammalari bo'yicha hisoblash masalasi ko'plab yetakchi olimlar V.V. Bolotin, I.I. Goldenblat, T.J. Junusov, I.L. Korchinskiy, A.M. Uzdin [20] va boshqa olimlar tomonidan muhokama qilingan. Ko'p tayanchli konstruksiyalarga tegishli ravishda zilzilalarning akselerogrammalari bo'yicha hisoblash ishlariga yondashuv A.Ter-Kyurigyaning fundamental ishida [8] tahlil qilib chiqilgan edi.

[8] ishda seysmik ta'sirga bajarilgan hisoblashda ixtiyoriy omil z kvazistik va dinamik tashkil etuvchilar ko'rinishida keltirilgan:

$$z(t) = \sum_{k=1}^N a_k u_k(t) + \sum_{k=1}^N \sum_{j=1}^{n_f} b_{jk} \xi_{jk}(t) \quad (1.1)$$

bunda $u_k(t)$ – k -tayanch siljishi;

$\xi_{jk}(t)$ – k-tayanchning qo'zg'alishida j-bosh koordinata qiymati;

a_k va b_{jk} – ta'sir koeffitsiyentlari;

N – inshoot tayanchlari soni;

nf – tebranishlarning hisobga olingan shakllari soni.

(1.1) formulasidagi birinchi qo'shiluvchi z qiymatning kvazistatik, ikkinchisi esa dinamik tashkil etuvchisini ifodalaydi.

Tayanchlarning siljishi u_k va bosh koordinatalari ξ_{jk} ga tasodifiy statsionar jarayon deb qaragan holda, A.Ter-Kyurigyan $z(t)$ tasodifiy jarayonning spektral zichligi G_{zz} va uning o'rtakvadratlik og'ishi σ_z ni quyidagi ko'rinishda ifodaladi:

$$G_{zz}(\omega) = \sum_{k=1}^N \sum_{s=1}^N a_k \cdot a_s \cdot G_{u_k u_s}(\omega) + 2 \sum_{k=1}^N \sum_{s=1}^{nf} a_k \cdot b_{sj} \cdot H_j(-i\omega) \cdot G_{u_k u_s}(\omega) + \sum_{k=1}^N \sum_{s=1}^{nf} \sum_{j=1}^{nf} b_{kj} \cdot b_{sj} \cdot H_j(i\omega) \cdot H_j(-i\omega) \cdot G_{u_k u_s}(\omega) \quad (1.2)$$

$$\sigma_z^2 = \sum_{k=1}^N \sum_{s=1}^N a_k \cdot a_s \cdot \rho_{u_k u_s} \cdot \sigma_{u_k} \cdot \sigma_{u_s} + 2 \sum_{k=1}^N \sum_{s=1}^{nf} \sum_{j=1}^{nf} a_k \cdot b_{sj} \cdot \rho_{u_k \xi_{sj}} \cdot \sigma_{u_k} \cdot \sigma_{\xi_{sj}} \quad (1.3)$$

Ushbu formulalarda G_{xu} orqali x va u jarayonlarning o'zaro spektral zichligi, N orqali esa tebranishlarning j-shakli uchun kompleksli uzatish funksiyasi ifodalanadi:

$$H(i\omega) = \frac{i}{\omega_j^2 - \omega^2 + 2i\zeta_j \omega_j \omega} \quad (1.4)$$

bunda ω_j , ζ_j – tegishli, tebranishlarning j-shakli bo'yicha chastotasi va so'nishi.

$\rho_{u_k u_s}$, $\rho_{u_k \xi_{sj}}$, $\rho_{\xi_{kj} \xi_{sj}}$ bilan korrelyatsiya koeffitsiyentlari belgilangan, ular spektral zichliklar orqali quyidagi integrallar yordamida ifodalanadi:

$$\rho_{u_k u_s} = \frac{1}{\sigma_{u_k} \sigma_{u_s}} \int_{-\infty}^{\infty} G_{u_k u_s}(i\omega) d\omega \quad (1.5)$$

$$\rho_{u_k \xi_{sj}} = \frac{1}{\sigma_{u_k} \sigma_{\xi_{sj}}} \int_{-\infty}^{\infty} H_j(-i\omega) \cdot G_{u_k u_s}(i\omega) d\omega \quad (1.6)$$

$$\rho_{\xi_{kj} \xi_{sj}} = \frac{1}{\sigma_{\xi_{kj}} \sigma_{\xi_{sj}}} \int_{-\infty}^{\infty} H_j(i\omega) \cdot H_j(-i\omega) \cdot G_{u_k u_s}(i\omega) d\omega \quad (1.7)$$

k va s nuqtalarda ta'sir korrelyatsiyasi darajasini tavsiflash uchun adabiyotlarda [8, 21] kogerentlik koeffitsiyenti yoki quyidagi ifoda orqali topiladigan kogerentlikning normallashtirilgan funksiyasi $\gamma(i\omega)$ tushunchasi kiritiladi:

$$\gamma(i\omega) = \frac{G_{\bar{u}_k \bar{u}_s}(i\omega)}{\sqrt{G_{\bar{u}_k \bar{u}_k}(i\omega) \cdot G_{\bar{u}_s \bar{u}_s}(i\omega)}} \quad (1.8)$$

Inshootlar zilzilabardoshligini hisoblashga bag'ishlangan tasodifiy funksiyalar nazariyasining ilovasi [47, 78] ishlarda yanada rivojlantirilgan. Bu ishlarda tenglamani ko'rib chiqilayotgan tasodifiy funksiyalarning o'rtacha qiymatida emas, balki eng katta qiymatlarida taqriban qo'llash imkoniyati asoslab berilgan. Bu esa CHSU doirasida yuklanishlarni jamlash uchun, (1.5)-(1.7) integrallardan foydalanib hisoblangan korrelyatsiya koeffitsiyentlarini qo'llashga imkon yaratadi.

Ko'rsatib o'tilgan yondashuv doirasida A.A. Petrov [22] yuguruvchi muzlagan to'lqin va ayrim tayanchlarning nokogerent qo'zg'alishi kabi ikki turdagi tasodifiy ta'sir ostida ko'p tayanchli konstruksiya tebranishlarini ko'rib chiqqan. Bunda u differensial harakat tenglamalarini boshlang'ich tizimi dempferlanmagan tizim tebranishlari shakliga ko'ra taqsimlash (mutanosib dempferlash) mumkin deb hisoblagan. Tebranishlar shakliga ko'ra noelastik qarshilik koeffitsiyentlari berilgan, deb hisoblangan edi.

3. Natija va muhokamalar

Endi, Seysmik yuklanishning turli xil spektral zichliklarida tebranish shakllarining korrelyatsiyasi masalasiga oid prof. V.A. Pshenichkina [23] tomonidan bajarilgan bundan keyingi tadqiqotlarda spektral zichlik yarim ekstremalligining hisobga olinishi 40 Gs dan ortiq chastotalarda, ya'ni aslida tovush tebranishlarida o'zini ko'rsata boshlashi aniqlandi. Bu zilzilabardoshlik nazariyasida qiymatni topish uchun A.A. Petrovning formulalaridan foydalanishga imkon beradi. Ammo yer yuzi nuqtalari tebranishlarining korrelyatsiyasini hisobga olish masalasi bundan ham murakkab.

Yer yuzi nuqtalari korrelyatsiyasiga bag'ishlangan masalaning eng mukammal tahlili [8] ishlarda keltiriladi. A. Ter-Kyurigyan ko'p tayanchli konstruksiya tayanchlari ostidagi qo'zg'alishlar sinxronligi va kogerentligining buzilishini keltirib chiqaruvchi to'rtta effektini ko'rsatib o'tadi.

Birinchi effekt seysmik to'lqinlar gruntida tarqalishining eng oxirgi tezligi bilan bog'liq (wave-passage effect). Ayni effekt yuguruvchi muzlagan to'lqin modeli yordamida tavsiflanib, ayrim tayanchlarning nosinxron tebranishlarini keltirib chiqaradi.

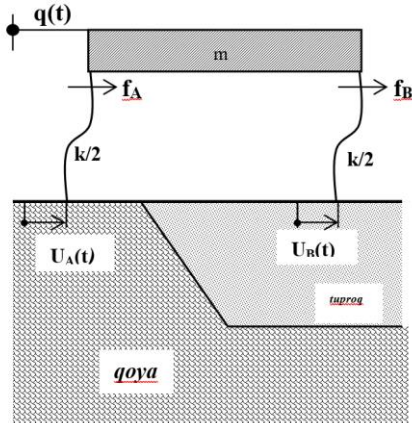
Ikkinchi effekt bir jinsli bo'lmagan grunt muhitida seysmik to'lqinlarning tarqalishi va difraksiyasi (incoherence effect) bilan bog'liq bo'ladi, bu esa yer yuzi nuqtalari tebranishlarining kogerentligi buzilishiga olib keladi. Bu effektning oqibatida boshlang'ich seysmik qo'zg'alishga asosning serqatlamligi, gruntida yoriqlar, linzalar va boshqa kiritmalarning bo'lishi bilan bog'liq yuqori chastotali tebranishlarning keng spektri qo'shiladi. Mazkur effekt eng muhim effektlardan biri bo'lib, ayrim ma'lumotlarga asosan bir-biridan 70 m dan ortiq masofada joylashgan nuqtalarning tebranishlarini nokogerent deb hisoblasha bo'ladi.

Uchinchi effekt zilzila episentridan uzoqlashgan sari seysmik to'lqinlarning so'nib borishi (attenuation effect) bilan bog'liq. Agar yuqori chastotali tebranishlar uzun davrli tebranishlarga nisbatan tezroq so'nib ketishi hisobga olinsa, zilzila manbasidan uzoqlashgan sari mazkur effekt ta'sirning spektral tarkibini o'zgartiradi. [6] da qayd etilganidek, bu effektini eng kam ta'sir etuvchi effekt deb hisoblasha bo'ladi, chunki u yer yuzi nuqtalari orasidagi masofa taxminan 1000 m va undan ortiq bo'lgan hollardagina namoyon bo'ladi.

To'rtinchi effekt bevosita har bir tayanchning turgan joyida grunt sharoitlari va profilning o'zgarishi bilan bog'liq bo'lib, "qurilish maydonchasi effekti" (site-response effect) deb nomlanadi. Bundan tashqari, mazkur effekt boshlang'ich seysmik ta'sirning o'zgarishiga ham ta'sir etish xususiyati bilan tarqalish effekti (incoherence effect) ga yaqin. Zilzila o'chog'idan to inshootgacha joylashgan gruntning kattagina qatlamini bir xil jinslardan iborat emasligi bilan bog'liq nokogerentlik effektidan farqli o'laroq qurilish maydonchasi effekti tayanchlar bevosita joylashgan zonadagi grunt qatlami xususiyatlarini hamda relyefning o'zgarishi bilan bog'liq. Shu sababli qurilish maydonchasi effekti to'lqinlarni tub tog' jinslaridan yer yuziga qadar tarqalishining sonli tekshiruvlari asosida baholanishi mumkin bo'ladi. Bu turdagi tadqiqot ishlari adabiyotlarda [24, 25 va b.] keng yoritilgan. Misollardan biri sifatida 2002-y. London shahrida o'tkazilgan XII Yevropa anjumanida taqdim etilgan [26] ishni ko'rsatib o'tish mumkin. Mualliflar bitta erkinlik darajasiga ega eng sodda tizimni o'rganib chiqishgan (3-rasm). Bu tizimda tayanchlar



har xil grunt sharoitlarida joylashgani sababli ularning qo'zg'alish darajasi turlicha bo'lgan.



3-rasm. Tayanchlarning turli tebranishlari sharoitida bir pog'onali erkin sistema

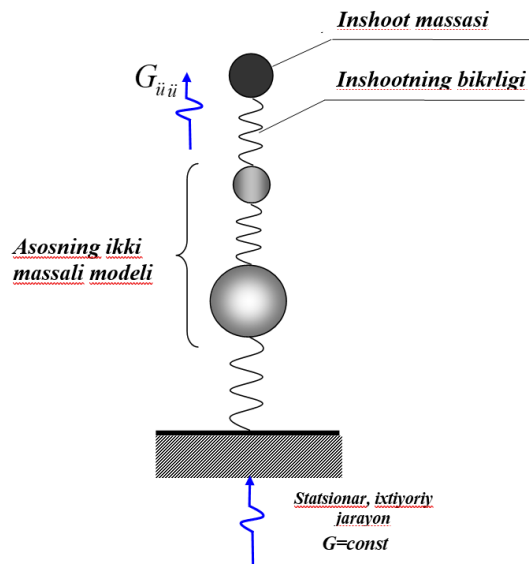
Mualliflar tizimga ikkita effektning: yuguruvchi muzlagan to'liqning (wave-passage effect) va har bir tayanch joylashgan yerda grunt sharoitlari o'zgarishining ta'sirini (site-response effect) tahlil qilishadi. Ikkinchi effekt tizimga kuchliroq ta'sir ko'rsatadi. Bunda mualliflar qatlam xususiyatlarini (elastiklik moduli va qalinligi), shuningdek qatlam va tub tog' jinslari akustik birliklarining o'zaro nisbatini tizimning o'zini tutishiga katta ta'sir ko'rsatishini qayd etib o'tadilar.

Mahalliy sharoitlar ta'sirida amalga oshirilgan tadqiqotlar ishlarini bajarish uchun tegishli seysmogeologik ma'lumotlar hamda vositalar mavjud bo'lganda qurilish maydonchasi effektini prognoz qilishga imkon yaratadi. Bundan tashqari, hozirgi vaqtda teskari masalani, ya'ni yer yuzining seysmik tebranishlari bo'yicha qurilish maydonchasi geologiyasini tiklash masalasini hal qilish imkonini mavjud. Y. Zaslavskiyning ishida [27] qurilish maydonchasidagi yer yuzi nuqtalari tebranishlarining o'zgarishiga oid mahalliy geologik sharoitlar prognozini tuzish uslubi ishlab chiqilgan.

Ta'sirning spektral zichligi[48]:

$$G_{\ddot{u}_k \ddot{u}_k}(\omega) = G_{kk} \cdot \frac{\omega_k^4 + 4 \cdot \zeta_k^2 \cdot \omega_k^2 \cdot \omega^2}{(\omega_k^2 - \omega^2)^2 + 4 \cdot \zeta_k^2 \cdot \omega_k^2 \cdot \omega^2} \cdot \frac{\omega^4}{(\omega_{pk}^2 - \omega^2)^2 + 4 \cdot \zeta_{pk}^2 \cdot \omega_{pk}^2 \cdot \omega^2} \quad (1.8)$$

Ta'sirning spektral zichligining (1.8) ko'rinishda ifodalanishi kirish signalining barqarorligini saqlashga va serqatlam asos tebranishlarining ko'proq uchraydigan davrlarini hisobga olishga imkon yaratadi. Ushbu jarayonning mexanik modeli 4-rasmda keltirilgan. Asos elementlari tebranish va so'nish parsial chastotalarining turli qiymatlaridan foydalangan va ularni shartli ravishda mustahkamligi har xil bo'lgan grunt asoslari bilan bog'lagan holda, asosni elastik dempirlash xususiyatlarining korrelyatsiya koeffitsiyentlariga ta'sirini sifatli baholashga erishish mumkin. Masalan, bunday tadqiqot ishlari bayoni [8] ishda keltirilgan.



4-rasm. (1.8) formula bo'yicha spektral zichlikni ifodalovchi mexanik model

Ikkinchi modeldan uning umumiyli tufayli tadqiqotlar amaliyotida juda ko'p foydalaniladi. Xususan, mazkur modelni A. Ter-Kiryugyan [8] va A.A. Petrovlar [26] o'zlarining asosiy ishlarida qo'llashgan. [22] ishda (1.9) formulaga qo'shimcha α_1 koeffitsiyenti kiritilganda, (1.10) formula quyidagi ko'rinishga keladi:

$$\gamma_{ks} = e^{-\left(\frac{\alpha \omega d_{ks}}{V_{sw}}\right)^2} \cdot e^{\left(i \cdot \frac{\omega d_{ks}^L}{V_{app}}\right)} \quad G_{\ddot{u}_k \ddot{u}_s}(i\omega) = \gamma_{ks}(i\omega) \sqrt{G_{\ddot{u}_k \ddot{u}_k}(\omega) \cdot G_{\ddot{u}_s \ddot{u}_s}(\omega)} \quad (1.9)$$

$$\gamma_{ks} = e^{-\left(\frac{\alpha \omega d_{ks}}{V_{sw}}\right)^2} \cdot e^{\left(i \cdot \frac{\alpha_1 \omega d_{ks}^L}{V_{app}}\right)} \quad G_{\ddot{u}_k \ddot{u}_s}(i\omega) = \gamma_{ks}(i\omega) \sqrt{G_{\ddot{u}_k \ddot{u}_k}(\omega) \cdot G_{\ddot{u}_s \ddot{u}_s}(\omega)} \quad (1.10)$$

Tayanchlar harakati butunlay korrelyatsiyalangan holatda:

$$\alpha = \alpha_1 = 0; \quad \chi_{ksjr} = 1 \quad (1.11)$$

Tayanchlar harakati statik erkin bo'lgan holatda:

$$\alpha \rightarrow \infty; \quad \alpha_1 = 0; \quad \chi_{kkjr} = 1; \quad \chi_{ksjr} = 0 \quad (k \neq s). \quad (1.12)$$

Yuguruvchi muzlagan to'liqin uchun

$$\alpha = 0; \quad \alpha_1 = 1; \quad \chi_{ksjr} = \frac{\omega_j + \omega_r}{2} \cdot \text{Re}(G_{ks}) = \cos\left(\frac{k_j + k_r}{2V_r} \cdot d_{ks}\right) \quad (1.13)$$

Umumiy hol uchun, α va α_1 parametrlarning ixtiyoriy qiymatlarida (1.5)-(1.7) integrallarga spektral zichlik uchun (1.10) ifodasini kiritgandan so'ng tegishli korrelyatsiya koeffitsiyentlarini sonli integrallashga va baholashga zarurat tug'iladi. Bunday hisoblashga doir tadqiqot ishlari [8] ishda bajarilgan. (1.10) spektral zichlikni dastlabki kiritishlari shartli xususiyatga ega va asosning haqiqiy mavjud xususiyatlariga bog'lanmagan bo'lsa ham, [8] ishda xulosalar muhim amaliy ahamiyatga ega. [8] da quyidagilar ko'rsatilgan:

1. Inshoot seysmik reaksiyasini psevdostatik tashkil etuvchilarining o'zaro korrelyatsiyasi juda katta bo'lishi mumkin va uni ko'p tayanchli konstruksiyalarni hisoblashda albatta hisobga olish kerak bo'ladi.

2. Inshoot seysmik reaksiyasi psevdostatik va dinamik tashkil etuvchilarining o'zaro korrelyatsiyasi kichik bo'lgani sababli, uni e'tiborga olmasa ham bo'ladi (har holda inshoot tebranishlari davri 2 sek dan kichik diapazonida ham).

3. Inshoot seysmik reaksiyasi psevdostatik tashkil etuvchilarining o'zaro korrelyatsiyasi ancha katta bo'lishi mumkin va uni inshootni hisoblashda e'tiborga olish kerak. O'rganilayotgan tebranishlar shakllarining xususiy chastotalari orasidagi farq kamayishi bilan tebranishlar shakli korrelyatsiyasining ta'siri oshib boradi.

4. Xulosa

O'rganilayotgan masala holatining tahlili inshootlarni seysmik ta'sirlarga mavjud hisoblash uslublari birinchi o'rinda konsolli sterjen ko'rinishida modellastiriluvchi nuqtali inshootlar uchun mo'ljallanganligini ko'rsatdi. Bunday inshootlar uchun ta'sir bitta nuqtaga – inshootning asosga tayanish nuqtasiga ko'rsatiladi.

Seysmik ta'sirlarga uzun inshootlar ishining tahlili ular shikastlanishlarining xususiyatlari asosni sinxron tarzda (nuqtali inshootlarda bo'lgani kabi) qo'zg'alishi haqidagi gipotezaga to'g'ri kelmasligini ko'rsatdi. Ularga inshoot uzunligi bo'yicha tezlanishlar maydonining bir xil bo'lmashligi, shuningdek amalda nosinxron tebranuvchi tayanchlarning bir-biriga nisbatan siljishi natijasida hosil bo'lgan shikastlanishlar sababli butun inshootda burovchi tebranishlarning vujudga kelishi xosdir.

Zilzilabardosh inshootlar loyihasini asoslashda ayni muammo muhim ahamiyat kasb etadi. Bu masalaga adabiyotlarda ancha katta e'tibor qaratilgan. Xususan, inshootlar tebranishlarining nosinxronlik sabablari aniqlandi va bunday inshootlarning tebranishlarining ham tasodifiy, ham determinik berilishida matematik modellarini qurishga doir jiddiy nazariy izlanishlar amalga oshirildi. Ammo ushbu uslublardan loyihalash amaliyotida foydalanib bo'lmaydi. Hozircha ular qurilish maydonchasida spektral egri chiziqlarning berilishidan kelib chiqadigan hisoblashlarning mavjud me'yoriy uslublari bilan bog'lanmagan va foydalanishdagi chiziqli-spektral uslubni takomillashtirish zarur.

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