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RESEARCH, INNOVATION, RESULTS



**TOSHKENT DAVLAT  
TRANSPORT UNIVERSITETI**

Tashkent state  
transport university



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# TASHKENT STATE TRANSPORT UNIVERSITY

## JOURNAL OF TRANSPORT

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## Development of the construction of the transition part in the zone of connection of the railway land line and the bridge, taking into account the effect of vibrodynamic forces

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**Abstract:** The article describes the design of the transition section for use in various conditions in the area of the junction of the roadbed and the bridge, the formation and causes of vertical shifts under the influence of vibro-dynamic forces that occur when trains move along the transition section. Also, in the areas of the junction of the roadbed and the bridge, various types of defects and their types are given, resulting from the fact that the pressure exerted by constant and temporary forces falling from the rolling stock exceeds the active ground pressure ( $E_a$ ) acting on the shore support of the bridge at the crossing site. In order to reduce the active pressure arising from the soil at the junction site, strengthen the junction site and prevent defects in the structures, methods for driving reinforced concrete piles into the embankment at the junction of the railway roadbed and the bridge and formulas for calculating the distances between the piles are given taking into account external influences.

**Keywords:** construction of the transition section, roadbed, bridge, embankment, soil, shore support, active ground pressure, reinforced concrete pile, dynamic rigidity

## Temir yo'l yer polotnosi va ko'prik tutashgan zonasida o'tish uchastkasi konstruksiyasini vibrodinamik kuch ta'sirini e'tiborga olgan holda ishlab chiqish

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**Annotatsiya:** Maqolada yer polotnosi va ko'prik tutashgan zonasida turli xil sharoitlarda foydalanish uchun mavjud o'tish uchastkasi konstruksiyasi, o'tish uchastkasida poyezdlarning harakati davomida hozir bo'ladigan vibrodinamik kuchlar ta'siridagi vertikal cho'kishlarning hosil bo'lishi va sabablari keltirilgan. Shuningdek, temir yo'l yer polotnosi va ko'prik tutashgan zonalarida yo'l ustki qurilmasidan tushadigan doimiy va harakatlanuvchi tarkibdan tushadigan vaqtincha kuchlar ta'siridagi bosim o'tish uchastkasidagi ko'priknig qirg'oq tayanchiga ta'sir qiluvchi gruntning faol bosimini ( $E_a$ ) ortib ketishi oqibatida turli xil nosozliklar paydo bo'lishi va ularning turlari keltirilgan. O'tish uchastkasida gruntndan paydo bo'ladigan faol bosimni kamaytirish, o'tish uchastkasini kuchaytirib, konstruksiyalarda hosil bo'ladigan nosozliklarni oldini olish maqsadida temir yo'l yer polotnosi va ko'prik tutashgan zonasidagi ko'tarmaga temirbeton qoziqlarni qoqish usullari va qoziqlar orasidagi masofalarni hisoblash formulalari tashqi ta'sirlarni e'tiborga olgan holda keltirib chiqarilgan.

**Kalit so'zlar:** o'tish uchastkasining konstruksiyasi, yer polotnosi, ko'prik, ko'tarma, grunt, qirg'oq tayanchi, gruntning faol bosimi, temirbeton qoziq, dinamik birlilik

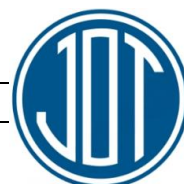
### 1. Kirish

Konstruksiyasi bo'yicha tayanchlar quyidagicha tasniflanishi mumkin: og'ir tayanchlar – toshli, butobetonli, betonli (monolit, yig'ma monolit yoki yig'ma). Ko'prik qurilishi davrining boshlanishida paydo bo'lgan va hozirgi vaqtgacha katta daryolarda intensiv muz ko'chish va boshqa murakkab sharoitlarda og'ir tayanchlar, hamda ustiga oraliq qurilmalar o'rnatiladigan sarrov (rigel) bilan tutashtirilgan, bitta yoki bir nechta qator qoziqlardan tashkil topgan konstruksiya ko'rinishidagi qoziqli tayanchlar qo'llanmoqda [1-2].

Qirg'oq tayanchlar ko'prik inshootlarining eng muhim elementlaridan bo'lib, oraliq qurilma konstruksiyasi va

kirish ko'tarmasi birikuvini ta'minlab beradi. Ko'priknig temir yo'l ko'tarmalari bilan birikuvini qirg'oq tayanchlar yordamida amalga oshiriladi. Bunday birikuvga qo'yiladigan asosiy talab – temir yo'l asosining birligi ravon o'zgarishi hisobiga ko'prikkas silliq kirib borish imkoniyatini yaratishdan iborat. Bunga, avvalambor, tayanch grunt vaznidan hamda tayanch orqasidagi ko'tarmaning vaqtincha yuklanishidan hosil bo'ladigan gorizontal bosimni o'ziga qabul qilib, ustki qismida katta vertikal siljishlarga yo'l qo'ymasligi orqali erishiladi. Bundan tashqari, birlikning o'zgarishi tayanch orqasida maxsus o'tish plitalarini yotqizish bilan ta'minlanadi. Ko'tarmaning oraliq tomonga surilib ketmasligi o'zi ham

 <https://orcid.org/0000-0002-1230-5552>





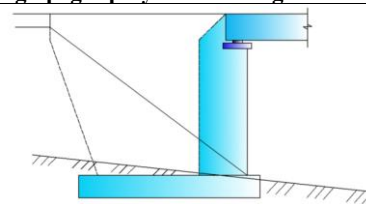
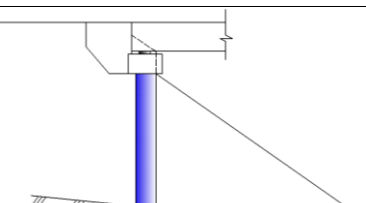
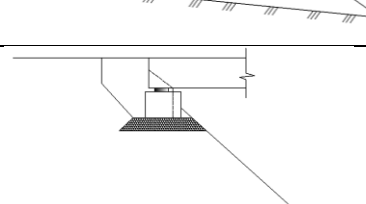
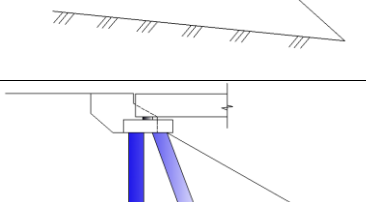
mustahkam bo'lishi kerak bo'lgan konus yordamida ta'minlanadi.

Metall va yog'och qoziqlardan iborat qoziqli tayanchlar vaqtincha ko'priklarda keng qo'llanmoqda. Qoziq qoqish texnikasi rivojlanishi bilan bunaqa tayanchlar temirbeton qoziqlardan barpo etilib, ulardan kichik suv havzalari ustida quriladigan doimiy ko'priklarda hamda yo'l o'tkazgichlarda foydalanilmoqda. 60-70-yillarda poydevorga tayangan va ustki qismi sarrov (rigel) yordamida biriktiriladigan, asosiy ko'taruvchi elementi temirbeton suyanchiqlar bo'lgan ustunsimon, hamda diametrlari 1.6 m va undan ortiq bo'lgan qobiqli qoziqlardan yoki burg'ulab qoqiladigan qoziqlardan iborat rostverssiz tayanchlar, monolit betondan yoki ko'pincha to'g'riburchakli kesimga ega bir-biriga yopishtirilgan va turli xil tipdagi poydevorlarga o'rnatilib, tepasi yaxlit kesimli temirbeton plita bilan tutashtiriluvchi beton bloklardan ishlanadigan ichi bo'sh tayanchlar kabi tayanchlarning xilma-xil turlari paydo bo'lib, ular tez orada quruvchilar tomonidan e'tirof etildi. Ichi bo'sh tayanchlar betonli (vertikal armaturasiz) yoki zo'riqtirilgan va zo'riqtirilmagan armaturali temirbeton tayanchlar ko'rinishida loyihalalanadi. Hozirda bunaqa ustunlar konstruksiyalarning asosiy turi bo'lib, eng samarali texnik yechimlar – qoziqli va ustunli tayanchlardan foydalanishga imkon beradi [3].

Hozirgi kunda o'tish uchastkalaridagi ko'priklarda foydalanib kelinayotgan qirg'oq tayanchlarning asosiy to'rtta turi 1-jadvalda keltirilgan.

1-jadval

## O'tish uchastkasidagi qirg'oq tayanchlarining turlari

|                                                                                |                                                                                     |
|--------------------------------------------------------------------------------|-------------------------------------------------------------------------------------|
| Qirg'oq tayanchlari (qiyalik qanothilari yoki tirgovuch devori ko'rinishidagi) |   |
| Qoziqli yoki ustun tipidagi yig'ma ustoy (bir qatorli)                         |  |
| Gorizontal to'sinli tayanch ko'rinishidagi ustoy                               |  |
| Qoziqli yoki ustun tipidagi yig'ma ustoy (ikki qatorli yoki chorpoyali)        |  |

## 2. Tadqiqot metodologiyasi

**Mavzuning dolzarbligi.** Bunda asosan qirg'oq tayanchi harakatlanuvchi tarkibning tebranish kuchi keltirib chiqaruvchi gruntning faol bosimini ( $E_a$ ) o'ziga qabul qilish va asosga uzatish, hamda inshootning xavfsiz ishlashini ta'minlashi lozim [4-6].

Gruntlarni to'siqlarga ko'rsatadigan bosim masalalari muhandislik hisob-kitoblarda eng muhimlaridan bo'lib, ular gruntlarning chekli zo'riqish holati nazariyasi va oldiga qo'yilgan masalalarni umumiy yechish usullari asosida topiladi: analitik, grafo-analitik, grafik.

Hozirgi vaqtda tirgovuch devorlari va boshqa to'siqlarga gruntlarning bosimi masalalariga bayoni va tahliliga ko'plab tomlardan iborat sermehnat ishlarning katta qismi (300 ortiq) bag'ishlangan. Bunaqa masalani oldimizga qo'ymasdan turib, gruntlarni to'siqlarga bosimi nazariyasining eng muhim qonunlarini, hamda ularni tirgovuch devorlariga ta'sir etuvchi grunt bosimini gruntlar mexanikasidagi eng yangi ma'lumotlar asosida hisoblash ishlariga tatbiq etilishini ko'rib chiqamiz.

Bularning barchasi ayni konstruksiyaning yechilishiga puxtalik bilan ilmiy yondashish hamda ko'priklarning qirg'oq tayanchini tutashish uzeli qo'shimcha tarzda hisoblash zaruratini keltirib chiqaradi.

Temir yo'l izlarining eng ko'p shikastlangan uchastkalari: Har bir transport tizimiga qo'yiladigan asosiy talablarning biri – bu ularning funksiyasidan kelib chiqqan holda ekspluatatsiya davrida ulardan foydalanishdir. Temir yo'l O'zbekiston Respublikasining asosiy transport tizimi bo'ladi. Temir yo'l transportini ishonchli va xavf-xatarsiz ishlashi uchun temir yo'lning asosiy qismlari bo'lib hisoblanadigan ustki va ostki qurilmalari, temiryo'l ko'prigiga tutash joylar me'yorlarga muvofiq saqlanishi kerak.

Qurilayotgan zamonaviy transport inshootlari tezyurur poyezdlarning harakat xavfsizligiga ta'sir etmasligi zarur. Bundan tashqari, temir yo'l ob'ektlarining ekspluatatsiyasini soddalashtirishga va xizmat muddatini oshirishga alohida e'tibor qaratish lozim.

Zilzila kuchlarining ta'siri natijasida temir yo'lning ustki va ostki qurilmasiga hamda ko'priklarga kirish joylariga ta'sir etuvchi kuchlarni ancha ko'p oshishi kuzatiladi, bu esa turli deformatsiyalarni kelib chiqishiga sabab bo'ladi.

Yer polotnosi gruntlarining xususiyatiga ko'ra asosiy maydonchasini kuchaytirishga doir tegishli ishlarni amalga oshirish kerak, aks holda bu zilzilalar hamda vibro-dinamik kuchlar ta'siri natijasida inshootlarni buzilishiga olib keladi.

Respublikamiz hududidagi, Toshguzar-Boysun-Qumqo'rg'on temir yo'l uchastkalaridagi temir yo'l va ko'priklarga tutashgan joylarida davriy takrorlanib turadigan temir yo'l izlarining vertikal cho'kishlarini xosil bo'lishi, buning natijasida yo'l ustki qurilmasi elementlariga shikastlar yetishi oqibatida poyezdlarning harakat tezligiga cheklavlar qo'yilishi bundan tashqari Qarshi-Termiz temir yo'lning Dehqonobod-Boysun uchastkasida temir yo'lga tosh tushishi bilan bog'liq salbiy ta'sirlarni ko'ramiz bu esa harakat xavfsizligiga o'z ta'sirini o'tkazadi.

O'tish uchastkalarida yer polotnosi gruntlarini tebranish jarayonlari: O'tish uchastkalarida temiryo'l yer polotnosini soz holatda saqlash yo'l sozlovchilardan juda katta mas'uliyat talab etuvchi ishlardan biridir. Yo'lning ayni shu joylarida ko'priklarga tayanchiga yaqinlashgan sari yer polotnosining deformatsiyalanuvchanlik xususiyati oshib ketadi. Deformatsiyalar miqdorini oshishi ko'tarmadan

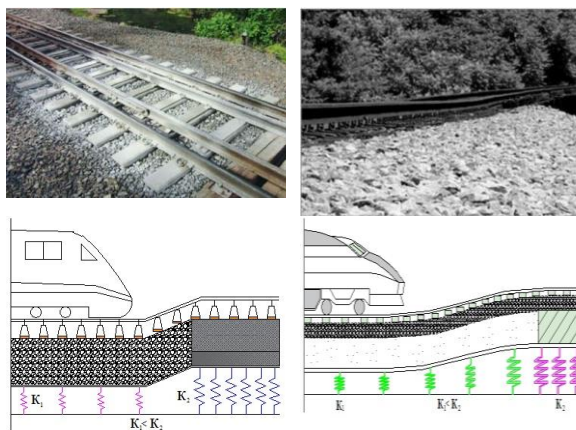
ko'priknining qatnov qismiga o'tish jarayonida yo'l bikrligini keskin darajada oshib ketishi kabi omillarga bog'liq.

Bikrligni bunday keskin darajada ko'tarilishi natijasida g'ildirakni relslarga bo'ladigan dinamik ta'siri ham oshadi. Bu esa, o'z navbatida, ballast qatlamida tebranishlarni kattalashishiga va oqibatda yer polotnosi gruntlariga ancha katta ta'sir ko'rsatadi. Yuqori darajali vibrodinamik ta'sirlar grunting mustahkamlik xususiyatlarini pasaytirib yuboradi, bu esa yer polotnosining ko'taruvchanlik qobiliyatini yo'qolib ketishiga olib kelishi mumkin.

Shu bilan birga ma'lumki, harakat tezliklari oshgan sari yer polotnosidagi tebranishlar amplitudalari harakat tezligiga to'g'ri proporsional tarzda oshadi. Poyezdning harakat tezligi oshishi bilan yer polotnosi jismla tebranishlar amplitudasi kattalashadi, buning natijasida yer polotnosining barqarorligi kamayadi.

Shunday qilib, tezyurar poyezdlarning harakatlanishi jarayonida vibrodinamik ta'sir darajasi ancha katta. Va bu, shubhasiz, ayniqsa o'tish uchastkalarida yo'l qo'yib bo'lmaydigan deformatsiyalarga olib keladi.

Tezyurar harakatdagi poyezdlarning dinamik yuklanishlari yo'lni ishdan chiqishiga katta ta'sir ko'rsatadi. O'tish zonasidagi rels izlari sifatini yomonlashuv jarayoni yo'l konstruksiyasi har xil (kirish joyidagi ballastli va ko'prikdagi ballastsiz) bo'lgan hollarda yaqqol ko'rinadi (1-rasm).

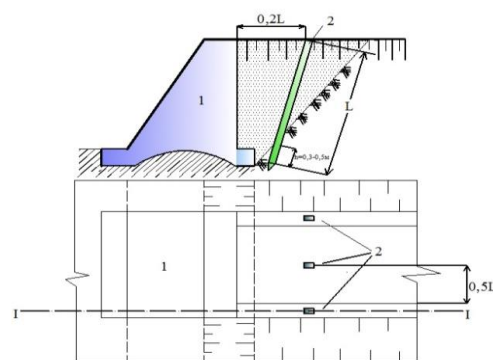


1-rasm. O'tish uchastkasida ballastni maydalanib ketishi natijasida ko'prikdagi chuqurni hosil bo'lishi

Ma'lum vaqt o'tishi bilan qirg'oq tayanchidan 1-3 m uzoqlikda tagida 8 mm li lyuftlar bo'lgan bir nechta shpalalar paydo bo'ladi. Lyuftlarning hosil bo'lishi yer polotnosi va ballastning namlik rejimini o'zgarishiga olib keladi (ayniqsa bahor faslida).

### 3. Natija va muhokamalar

**Material tahlili.** Yer polotnosini ko'priknining qirg'oq tayanchi oldidagi ko'tarma qismini qoziqlar yordamida mustahkamlash grunting inshootga faol bosimi ta'sirini kamaytiradi, hamda harakatlanuvchi transportdan tushadigan zarba kuchlarining bir qismi bevosita asos gruntiga uzatiladi. Qoziqlarni qoqishda ko'tarma grunting qo'shimcha tarzda zichlanadi, va bu kuchlanishlarni inshoot kesimiga yanada tekisroq taqsimlanishiga olib keladi (2-rasm).



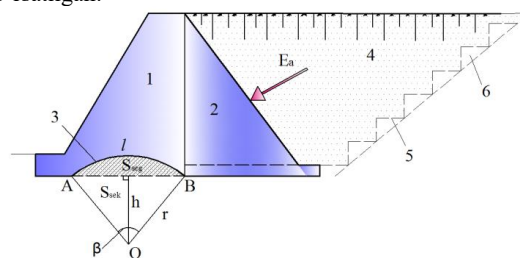
2-rasm. Ko'priknining qirg'oq tayanchi oldidagi (o'tish uchastkasida) yer polotnosining ko'tarma qismini qoziqlar bilan mustahkamlash: 1 – ko'priknining qirg'oq tayanchi; 2 – temirbeton qoziqlar; L – qoziqlar uzunligi

Poyezdlarni tezyurar harakati liniyalarida yer polotnosining nisbatan zaif grunt asoslarida joylashgan baland ko'tarmalar barqarorligini ta'minlanishiga oid shunga o'xshash usul ishlab chiqib keltirib o'tilgan.

1. Yer polotnosini mustahkamlashda, qoziqlardan foydalanish butun inshootning zilzilarga qarshi barqarorligini ta'minlaydi, chunki bu konstruksiya nisbatan elastik bo'lib, ko'tarma gruntingni buzilib ketmasdan deformatsiyalanishiga imkon yaratadi (bu haqida tegishli ma'lumot batafsil ishda keltirilgan).

2. Eng samarali temirbeton qoziqlar – bu qo'shtavrlar qoziqlardir, ular to'g'riburchakli qoziqlarga nisbatan ancha yengil va grunt bilan tutashish maydoni katta bo'ladi. Bir izli temir yo'llar uchun – 2 ta, ikki izlilar uchun esa 3 ta qoziqlarning o'zi yetarli.

Qirg'oq tayanchining botiq holatida ishqalanish kuchi oshadi, shu bilan uning barqarorligi ta'minlanadi. Matematik nuqtai nazardan qirg'oq tayanchning botiq asosini loyihalashda ishqalanish yuzasini ortishi 3-rasmida ko'rsatilgan:



3-rasm. Qirg'oq tayanchining botiq asosini grunt bilan qo'shimcha kesishish maydonini hisoblash: 1 – tayanch devor; 2 – kontroforslar; 3 – tirgag devorning botiq tayanch yuzasi; 4 – yo'lning yer polotnosi; 5 – o'yiqlar; 6 – ko'chish grunting massasi; Ea – grunting faol bosimi

Botiq tayanchni doiraviy sektor va segmentga ajratamiz. Sektor maydoni quyidagi ifodadan topiladi:

$$S_{\text{sek}} = \frac{\pi r^2 \beta}{360^\circ}, \quad (1)$$

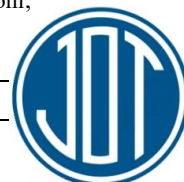
Segment maydoni quyidagi ifodadan topiladi:

$$S_{\text{seg}} = \frac{\pi r^2 \beta}{360^\circ} - S_{\text{AOB}}, \quad (2)$$

bu yerda  $r$  – aylana radiusi;

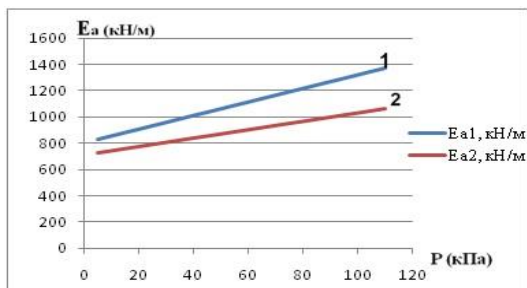
$\beta$  – markaziy burchak;

$S_{\text{AOB}}$  – uchburchakning sektordagi maydoni;



$\pi=3,14$ .

Temir yo'l yer polotnosi va ko'priklarni tutashgan zonasidagi ko'tarmaga temirbeton qoziq qoqish orqali ko'priklarni qirg'oq tayanchiga ko'tarma gruntning faol bosimini tarqalishi grafidga ifodalaymiz (4-rasm).



4-rasm. Gruntning faol bosimini bir tekis taqsimlangan kuchga bog'liqligi: 1 – gruntning qirg'oq tayanchiga ta'sir etuvchi faol bosimi; 2 – qoziqlar qoqilgandan so'ng gruntning qirg'oq tayanchiga ta'sir etuvchi faol bosimi

Ko'priklarga yaqinlashuv joyida gruntlarning dinamik birligini oshirish maqsadida ko'tarma balandligi 7 m bo'lganda, gilli gruntlarda qoqiladigan temirbeton qoziqlar orasidagi masofani aniqlaymiz: I izli uchastkalarda yer polotnosi asosiy maydonchasi kengligi 7,6 m, II izli uchastkalarda asosiy maydoncha kengligi 11,7 (ShNK 2.05.01-19. 1520 mm izli temir yo'llar. Loyihalash me'yorlari).

$$L = \frac{B^2 \cdot K}{H \cdot \tan \varphi} + h \cdot \tan \alpha; \quad (3)$$

$$L = \frac{7,6^2 \cdot 1,0}{7 \cdot \tan 20} + 7,74 \cdot \tan 20 = 25,7 \text{ m.}$$

bunda  $L$  – temirbeton qoziqlar orasidagi masofa, m;

$B$  – asosiy maydoncha kengligi,  $B=7,6$  m;

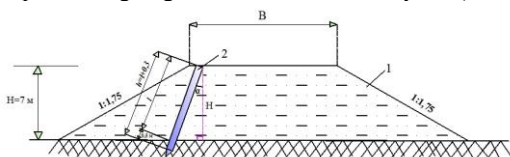
$H$  – ko'tarma balandligi,  $H=7$  m;

$\varphi$  – grunt xususiyatlarini hisobga oluvchi dinamik koeffitsiyent, Gilli gruntlar uchun  $\varphi=20$ ;

$K$  – poyezdlarning harakat tezligini hisobga oluvchi koeffitsiyent;

$\alpha$  – qoziqlarning og'ish burchagi;  $\alpha=20^\circ \div 40^\circ$  oraliqda bo'lishi maqsadga muvofiq  $\alpha = 20^\circ$ .

$h$  – temirbeton qoziqlar uzunligi, m. Temirbeton qoziqlar uzunligini geometrik usulda hisoblaymiz (5-rasm).



5-rasm. Temirbeton qoziq uzunligini aniqlash sxemasi: 1 – temir yo'lining yer polotnosi; 2 – temirbeton qoziq

$$\cos \alpha = \frac{H}{l}, \text{ bundan } l = \frac{H}{\cos \alpha}.$$

$$h = l + 0,3 = \frac{H}{\cos \alpha} + 0,3 = \frac{7}{\cos 20} + 0,3 = 7,74 \text{ m.}$$

Gilli gruntдан barpo qilingan I izli uchastkada ko'tarmaga qoqilgan temirbeton qoziqlar orasidagi hisoblangan masofalar 2-jadvalda keltirilgan.

2-jadval

Gilli gruntдан barpo qilingan ko'tarmaga qoqiladigan temirbeton qoziqlar orasidagi masofa

|                                                                                  |          |
|----------------------------------------------------------------------------------|----------|
| I izli uchastkada<br>ko'tarma balandligi 7 m,<br>asosiy maydoncha kengligi 7,6 m |          |
| $L=12,6$ m                                                                       | $K=1,0$  |
| $L=12,1$ m                                                                       | $K=0,95$ |
| $L=11,6$ m                                                                       | $K=0,90$ |
| $L=11,1$ m                                                                       | $K=0,85$ |
| $L=10,6$ m                                                                       | $K=0,80$ |
| $L=10,4$ m                                                                       | $K=0,78$ |

Gilli gruntдан barpo qilingan II izli uchastkada ko'tarmaga qoqilgan temirbeton qoziqlar orasidagi hisoblangan masofalar 3-jadvalda keltirilgan.

3-jadval

Gilli gruntдан barpo qilingan ko'tarmaga qoqiladigan temirbeton qoziqlar orasidagi masofa

|                                                                                    |          |
|------------------------------------------------------------------------------------|----------|
| II izli uchastkada<br>ko'tarma balandligi 7 m,<br>asosiy maydoncha kengligi 11,7 m |          |
| $L=26,1$ m                                                                         | $K=1,0$  |
| $L=24,9$ m                                                                         | $K=0,95$ |
| $L=23,7$ m                                                                         | $K=0,90$ |
| $L=22,6$ m                                                                         | $K=0,85$ |
| $L=21,4$ m                                                                         | $K=0,80$ |
| $L=20,9$ m                                                                         | $K=0,78$ |

## 4. Xulosa

1. Nazariy hisob-kitoblarga asoslangan holda yuqoridagi grafikdagi 80 kPa bosimdagi nuqtada, o'tish uchastkasiga tavsiya etilayotgan qoziqlarni qoqish natijasida ko'priklarning qirg'oq tayanchiga grunt qatlamini o'z vazni hamda harakatlanuvchi tarkibning vibrodinamik kuchi natijasida vujudga keladigan faol bosim ta'sirini 23% ga kamaytirishga erishiladi.

2. Elastikli yuqori bo'lgan temirbeton qoziqlarni ko'tarmaga qoqish orqali yer polotnosini dinamik birligi ortadi, grunt zichlanadi va gruntning ichki ishqalanish burchagi esa sezilarli ortishi aniqlandi. Buning natijasida o'tish uchastkasida yer polotnosi asosiga vertikal yuklamalarning 25% ni o'tkazilishi hisobiga, yer polotno ko'tarmasi gruntida zo'riqish zonasining chuqurligi kamayadi.

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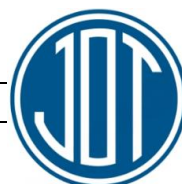
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